

Appendix E – Ground Investigation Report

rpsgroup.com Page 30

6th October 2022

Our ref: GE21162/SA01/221006



Oliver Troup Statera Energy 145 Kensington Church Street London W8 7LP

By email

Dear Oliver,

RE: Land to the rear of Culham Science Centre, Culham, Abingdon, OX14 3DB - Letter Report

1. Introduction

Further to your instruction, we write to present the findings of the intrusive investigation undertaken at land to the rear of Culham Science Centre, Culham, Abingdon, OX14 3DB (Figure 1).

2. Proposed Development

The proposed development is understood to comprise a battery energy storage facility with associated infrastructure including a number of stormwater basins.

3. Objectives

The investigation was undertaken to inform the emerging drainage strategy for the proposed development.

4. Site Description

The site was located at NGR 452888, 196448 and was formed by an irregularly shaped parcel of land comprising sections of two grazing fields. The fields were separated by a concrete track initially running east to west in the very south of the site, before turning to run in a north-north-east direction through the centre of the site. Topographically, the site sloped gently downhill to the south.

Two strings of overhead electricity cables supported by pylons ran across the site. One string ran north to south through the western part of the site with one supporting pylon located in the northwest of the site. The second string of cables ran along the south-eastern boundary before crossing the eastern extend of the site. Two inspection chamber covers were observed in the northwest of the site to the south of the pylon in this area. When lifted, these appeared to be former soakaways. A trunk water main was mapped running southwest to northeast through the east of the site. Although no visible evidence of this utility was recorded, it was possible to trace its location using radio-detection techniques during the investigation.

The majority of the site boundaries did not coincide with any physical feature with the exception of the western boundary which was formed by post and wire fencing and a deciduous hedgerow. Access to the site was afforded by an external perimeter track around the Science Park accessed via the industrial estate to the southwest of the site.

A railway line within a cutting was located to the west of the site. A continuation of grazing fields, some woodland and cover crop (maize) were located to the north and northeast. A raised area covered by long grass with a number of deciduous trees was located to the east with a large warehouse type building approximately 200m beyond. An electricity pylon and small mobile telecommunications compound, surrounded by timber close-boarded fencing was located immediately adjacent to the southeast of the site with Culham Science Park beyond. The Science Park was secured by tall metal mesh fencing. Two further electricity pylons were located to the south

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of the site in an area of long grass. One of these was noted to connect to a large substation located within the Science Park.

5. Fieldwork

The scope of works agreed with the Client comprised:

- Attendance of a Geo-Environmental Engineer to set out and supervise the intrusive investigation, undertake logging of recovered soils from exploratory holes and in-situ testing.
- Construction of 3No. dynamic windowless sampler boreholes (WS1 to WS3) to depths of up to 5.00m bgl.
- Installation of the boreholes with 3No. monitoring standpipes with upstanding covers to allow for future groundwater monitoring.
- Construction of 4No. machine excavated trial pits (TP1 to TP4) to depths between 1.60m and 2.00m bgl.
- Soakage testing in accordance with BRE365 undertaken in trial pits TP1 to TP4.
- 3No. return groundwater monitoring visits.
- Provision of a Letter Report.

The intrusive investigation was carried out on 27th, 28th and 29th September 2022. The positions were agreed with the Client but adjusted on site to avoid overhead and underground utilities. The locations of the exploratory holes are shown on Figure 2.

6. Ground Conditions

The ground conditions encountered by the investigation comprised a mantle of Topsoil overlying Summertown-Radley Sand and Gravel Member. A generalised summary of the encountered conditions is presented in Table 1.

Top (m bgl)	Base (m bgl)	Geology	Locations
0.00	0.20 - 0.35	TOPSOIL: Brown or light brown fine and medium SAND with low or moderate proportions of silt and gravel with occasional or some rootlets.	All
0.20 - 0.35	>1.60 ->5.00	summerrown-radier sand and gravel member: Typically orange brown, but occasionally brown or light orange brown, medium or medium and coarse SAND with low to moderate silt and gravel content. Moderate clay content occasionally recorded. Silt and gravel also absent on occasion. Greenish grey and grey colouration, some decayed 5-10mm roots and natural organic odour recorded from 1.00m to 1.70m bgl in TP4.	All

Table 1 Summary of Ground Conditions

For further details of the ground conditions encountered, reference should be made to the exploratory hole logs in Appendix A.

8. Groundwater

Groundwater was encountered within WS2 at 3.18m bgl immediately after drilling, rising to 3.30m bgl with the installed monitoring standpipe after 3.5 hours. Groundwater was recorded at 4.95m bgl in WS3 immediately after drilling but had dropped to a depth of greater than 5.00m bgl in the monitoring standpipe after 2.5 hours. No other groundwater was encountered during the intrusive investigation.



















Groundwater monitoring standpipes were installed within WS1 to WS3 in order to facilitate return spot monitoring of groundwater levels . 3No. groundwater monitoring visits are planned, upon completion this report will be updated to include the data collected.

The groundwater level in the 3No. monitoring standpipes was measured on 29th September 2022 (the final day of the investigation and two days following installation). No groundwater was recorded within the depth of the installed standpipes with the exception of WS2 where the water depth was recorded at 3.01m bgl.

It should be noted that changes in groundwater and perched water levels do occur for a number of reasons including seasonal effects and variations in drainage. Such fluctuations may only be recorded by the measurement of the groundwater level within a series of standpipes or piezometers installed within appropriate response zones.

9. Obstructions

Boreholes WS1 and WS2 refused on dense strata at depths of 3.70m and 4.70m bgl respectively. No other natural or manmade obstructions were encountered. Obstructions elsewhere on the site cannot be completely ruled out.

10. Soakaways

Soakage testing in broad accordance with BRE365 was undertaken in trial pits TP1 to TP4. Testing was undertaken on 28th and 29th September 2022 for the two day test period agreed with the Client.

The results of the testing are summarised in Table 2 below. The soakage test results are included within Appendix B. In trial pits TP1 and TP3 Test 1 was abandoned, and Test 2 commenced at the end of the first day of testing when it became apparent that Test 1 would conclude during the night when accurate measurement would not be possible.

	Pit		Permeability (m/s)	
Location	depth (m bgl)	Test 1	Test 2	Test 3
TP1	1.70	2.6 x 10 ⁻⁶ *	2.6 x 10 ⁻⁶	1.9 x 10 ⁻⁶ *
TP2	1.90	1.4 x 10 ⁻⁵	1.1 x 10 ⁻⁵	8.7 x 10- ⁻⁶
TP3	1.60	3.3 x 10 ⁻⁶ *	2.9 x 10 ⁻⁶	2.8 x 10 ⁻⁶ *
TP4	2.00	2.7 x 10 ⁻⁶	Insufficient time to co	omplete further tests

NOTE: * - based on data extrapolation

Table 2 Soakage Test Results

In line with building control requirements soakaways should be located at least 5m from any structure.

11. Conditions

The data collected from the investigations have been used to provide an interpretation of the geotechnical and/or environmental conditions pertaining to the site. The recommendations and opinions expressed in this report are based on the data obtained. Geo-Environmental takes no responsibility for conditions that either have not been revealed in the available records, or that occur between or under points of physical investigation. Whilst every effort has been made to interpret the conditions, such information is only indicative and liability cannot be accepted for its accuracy.

A Discovery Strategy (Appendix C) should remain in force throughout groundworks and construction of the proposed development.



















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12. Closure

We trust that we have interpreted your instructions correctly. Please do not hesitate to contact us should you have any queries.

Yours sincerely For and on Behalf of Geo-Environmental

SHAUN ARMITAGE BSc (Hons), FGS Principal Consulting Engineer

shaun.armitage@gesl.net

Enclosed - Figure 1 – Site Location Plan

Figure 2 – Exploratory Hole Location Plan

Appendix A – Exploratory Hole Logs Appendix B – Soakage Test Results Appendix C – Discovery Strategy







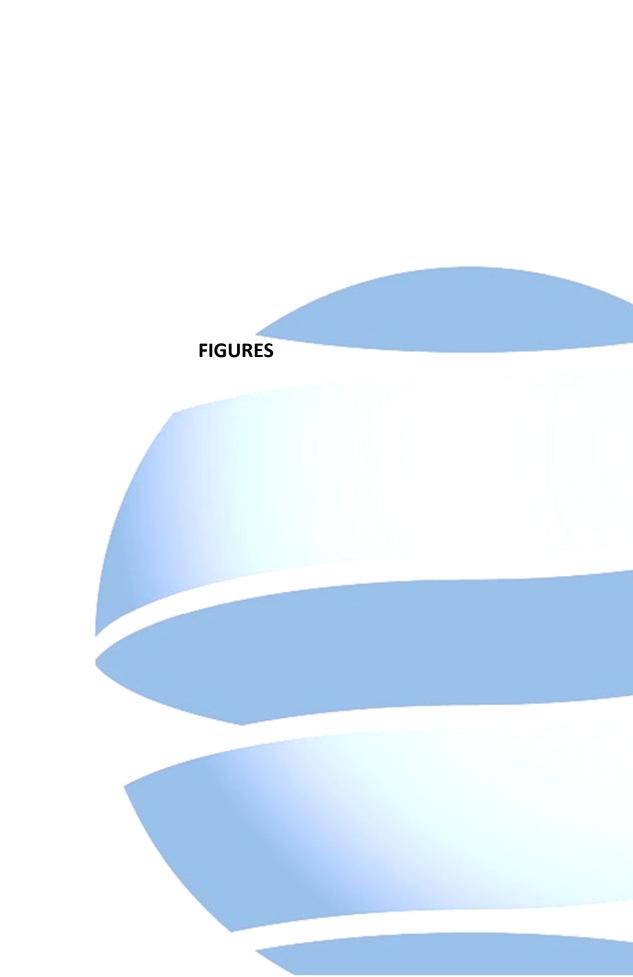














Project:	Culham, Abingdon			Title
Client:	Statera Energy			
Ref No:	GE21162	Version:	0.0	
Drawn:	SA	Date:	05/10/2022	
Figure:	1	Scale:	Not To Scale	

Site Location Plan

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Project Title: Culham, Abingdon

Title: Figure 2 - Exploratory Hole Location Plan

Location: Oxfordshire, OX14 3DB

Scale: 1:3000

Project No.: GE21162

Engineer: CG

Client: Statera Energy

Geo-Environmental



Locations By Type - Empty

Locations By Type - TP

Locations By Type - WLS



APPENDIX A Exploratory Hole Logs

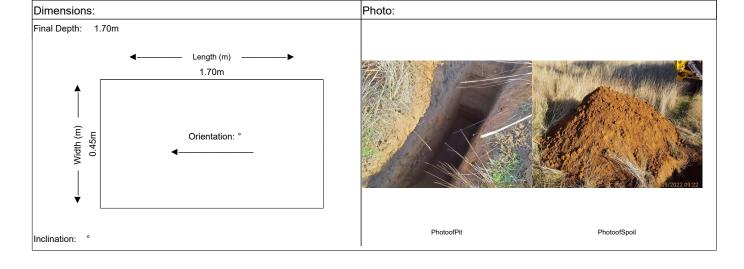
Borehole No. Unit 7, Danworth Farm Hurstpierpoint **Borehole Log WS1** BN6 9GL Geo-Environmentalwww.gesl.net Sheet 1 of 1 Project No. Hole Type 453010E - 196474N Culham, Abingdon Co-ords: Project Name: GE21162 WLS Scale Location: Oxfordshire, OX14 3DB Level: 1:25 Logged By 27/09/2022 Client: Statera Energy Dates: SA Sample and In Situ Testing Water Depth Level Well Legend Stratum Description (m) Strikes (m) Depth (m) Type Results Light brown silty slightly gravelly fine and medium SAND with some rootlets. TOPSOIL 0.20 Orange brown slightly silty slightly gravelly medium SAND. Gravel is medium and coarse subrounded quartzite and various lithologies. SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER 0.90 Orange brown slightly gravelly medium and coarse SAND. Gravel is fine subrounded quartzite. SUMMERTOWN-RADLEY SAND AND GRAVEL 1.20 Orange brown medium and coarse SAND. Trace of SUMMERTOWN-RADLEY SAND AND GRAVEL 2.20 Orange brown slightly silty slightly gravelly medium and coarse SAND. Gravel is fine subrounded quartzite. SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER 3 3.70 End of Borehole at 3.70m Dynamic Sampling Run Details Water Strike Details (mbgl) Remarks epth Top Depth Base Diameter Depth Strike Rose To Refused at 3.7m. No groundwater encountered. No groundwater in standpipe after 1.5 hours. No groundwater in

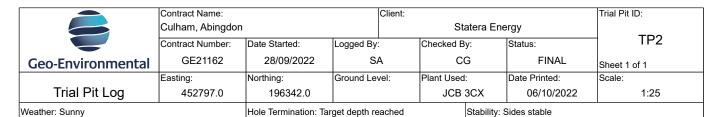
Borehole No. Unit 7, Danworth Farm Hurstpierpoint **Borehole Log** WS2 BN6 9GL Geo-Environmentalwww.gesl.net Sheet 1 of 1 Project No. Hole Type Co-ords: 452896E - 196269N Culham, Abingdon Project Name: GE21162 WLS Scale Location: Oxfordshire, OX14 3DB Level: 1:25 Logged By 27/09/2022 Client: Statera Energy Dates: SA Sample and In Situ Testing Water Depth Level Well Legend Stratum Description (m) Strikes (m) Results Depth (m) Type Brown slightly silty medium SAND with some rootlets. 0.30 Light orange brown gravelly medium SAND. Gravel is fine occasionally coarse subrounded quartzite and various lithologies. SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER 1.00 Orange brown gravelly medium SAND. Gravel is fine occasionally coarse subrounded quartzite and various lithologies SUMMERTOWN-RADLEY SAND AND GRAVEL **MEMBER** Ground wet from 2.6m 3 3.10 Orange brown slightly silty gravelly medium SAND. Gravel is fine occasionally coarse subrounded quartzite and various lithologies. SUMMERTOWN-RADLEY SAND AND GRAVEL **MEMBER** 4.70 End of Borehole at 4.70m Dynamic Sampling Run Details Water Strike Details (mbgl) Remarks epth Top Depth Base Diameter Rose To Depth Strike Refused at 4.7m. Groundwater at 3.18m after drilling. Groundwater at 3.30m in standpipe after 3.5 hours. Groundwater 3.18 3.30

Borehole No. Unit 7, Danworth Farm Hurstpierpoint **Borehole Log** WS3 BN6 9GL Geo-Environmentalwww.gesl.net Sheet 1 of 1 Project No. Hole Type 452728E - 196561N Culham, Abingdon Co-ords: Project Name: GE21162 WLS Scale Location: Oxfordshire, OX14 3DB Level: 1:25 Logged By 27/09/2022 Client: Statera Energy Dates: SA Sample and In Situ Testing Water Depth Level Well Legend Stratum Description (m) Strikes (m) Depth (m) Type Results Brown slightly silty gravelly medium SAND with some **TOPSOIL** 0.25 Orange brown occasionally brown silty gravelly medium SAND. Gravel is fine subrounded quartzite and various lithologies SUMMERTOWN-RADLEY SAND AND GRAVEL **MEMBER** 1.20 Orange brown slightly silty medium SAND. SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER 2.00 Orange brown clayey medium SAND with one coarse 2.10 subrounded quartzite gravel clast. SUMMERTOWN-RADLEY SAND AND GRAVEL **MEMBER** Orange brown slightly silty medium SAND. SUMMERTOWN-RADLEY SAND AND GRAVEL **MEMBER** 2.80 Orange medium and coarse SAND. SUMMERTOWN-RADLEY SAND AND GRAVEL 5.00 End of Borehole at 5.00m Dynamic Sampling Run Details Water Strike Details (mbgl) Remarks epth Top Depth Base Diameter Rose To Depth Strike Groundwater at 4.95m after drilling. No groundwater in standpipe after 2.5 hours. No groundwater in standpipe on 29 4.95 4.95

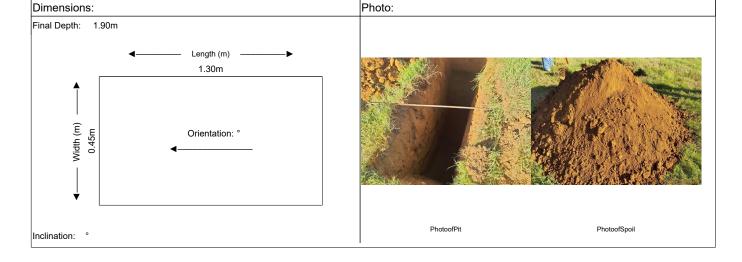
	Contract Name:			Client:				Trial Pit ID:
	Culham, Abingdon	Statera Energy				,		
Contract Number:		Date Started: Logged By:		Checked By:		Status:	TP1	
Geo-Environmental	GE21162	28/09/2022	S	A	C	G	FINAL	Sheet 1 of 1
	Easting:	Northing:	Ground Le	vel:	Plant Used	:	Date Printed:	Scale:
Trial Pit Log	452866.0	196274.0			JCB	3CX	06/10/2022	1:25
Weather: Sunny		Hole Termination: Tar	get depth re	eached		Stability: 8	Sides stable	

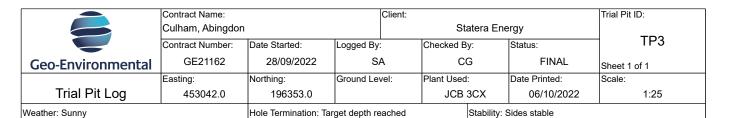
Sa	amples & In	Situ Testing		Strata Details							
Depths	Sample ID	Test Result	Reduced Level	Level (Thickness) Legeria Strata Description							
				(0.30)		Brown slightly slity slightly gravelly medium SAND with occasional rootlets. TOPSOIL	-				
				0.30		Orange brown slight silty slightly gravelly medium SAND. Gravel is fine occasionally coarse subrounded quartzite and various lithologies. SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER	- 0.5 				
				(1.40)			1.0 1.5				
				1.70	- 	End of Trial Pit at 1.70m					
							_ _ _ _ 2.5				
							-				
							3.0				
							— 3.5 —				



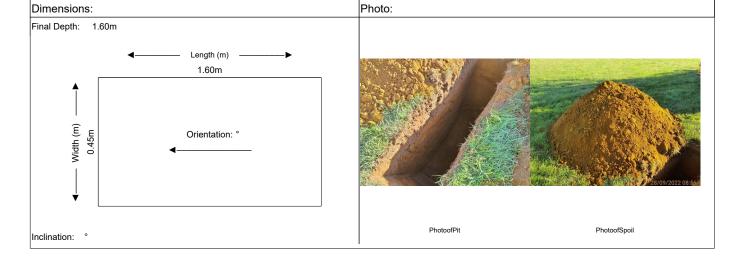


Sa	mples &	In Situ Testing		Strata Details							
Depths	Sample ID	Test Result	Reduced Level	Depth (m) (Thickness)	Legend	Strata Description		Water	Backfi		
				(0.35)		Brown slightly silty slightly gravelly medium SAND with some rootlets. TOPSOIL	_				
				0.35		Orange brown silty slightly gravelly medium SAND. Gravel is fine and medium subrounded quartzite and various lithologies. SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER					
				(0.75)	× × × × × × × × × × × × × × × × × × ×		_ _ _ _ 1.0				
				1.10	* * * * * * * * * * * *	Brown slightly silty medium SAND with rare medium subangular sandstone and subrounded quartzite gravel.	_				
				(0.40)	×	SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER	_ _ _ 1.5				
				(0.40)		Orange brown medium SAND with rare medium subrounded quartzite gravel. SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER	- -				
				1.90		End of Trial Pit at 1.90m	2.0				
							_				
							_ — 2.5				
							_				
							3.0				
							- - -				
							_ — 3.5				
							<u> </u>				



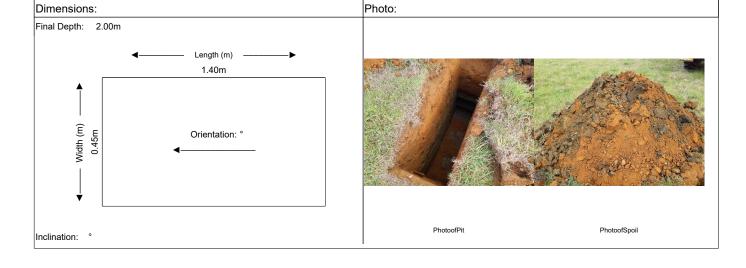


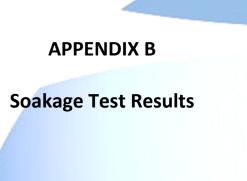
Sa	mples &	In Situ Testing				Strata Details		Water	Backfill
Depths	Sample ID	Test Result	Reduced Level	Depth (m) (Thickness)	Legend	Strata Description		×	Dackilli
				(0.30)		Brown slightly slity slightly gravelly medium SAND with occasional rootlets. TOPSOIL			
				0.30		Orange brown medium SAND with a trace of silt. SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER	- - 0.5		
				(0.90)			_ _ _ 		
				1.20		Orange brown slightly gravelly medium SAND. Gravel is fine subrounded quartzite. SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER	_		
<u>-</u> -				1.60		End of Trial Pit at 1.60m	— 1.5 ——		
· · ·									
							-		
-							_ — 2.5 —		
							-		
							3.0 		
							- - - 3.5		
							- 3.5 -		



	Contract Name:			Client:				Trial Pit ID:
	Culham, Abingdon	Statera Energy				TD 4		
	Contract Number:	Date Started:	Logged By		Checked B	y:	Status:	TP4
Geo-Environmental	GE21162	28/09/2022	S	Ą	C	3	FINAL	Sheet 1 of 1
	Easting:	Northing:	Ground Le	/el:	Plant Used	:	Date Printed:	Scale:
Trial Pit Log	452825.0	196462.0			JCB	3CX	06/10/2022	1:25
Weather: Sunny		Hole Termination: Tar	get depth re	ached		Stability: S	Sides stable	

Sa	mples & I	n Situ Testing				Strata Details		Water	Backf
Depths	Sample ID	Test Result	Reduced Level	Depth (m) (Thickness)	Legend	·			Backfill
				(0.30)		Brown slightly silty slightly gravelly medium SAND with some rootlets. TOPSOIL	_		
				0.30	X	Orange brown slightly gravelly medium and coarse SAND. Gravel is fine subrounded quartzite. SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER	_ _ _ 0.5		
				(0.70)			- -		
				1.00		Greenish grey and grey brown clayey medium SAND with some decayed 5-10mm roots. Slight natural organic odour. SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER	1.0 		
				(0.70)		SOWINIER TOWN-RADLET SAND AND GRAVEL MEMIDER	- -		
				1.70	~ × ×	Orange brown silty medium and coarse SAND.	— 1.5 —		
				(0.30)	× × × × × × × × × × × × × × × × × × ×	SUMMERTOWN-RADLEY SAND AND GRAVEL MEMBER End of Trial Pit at 2.00m	2.0		
							- - -		
							- - 2.5		
							- -		
							3.0 		
							-		
							- 3.5 -		





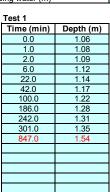


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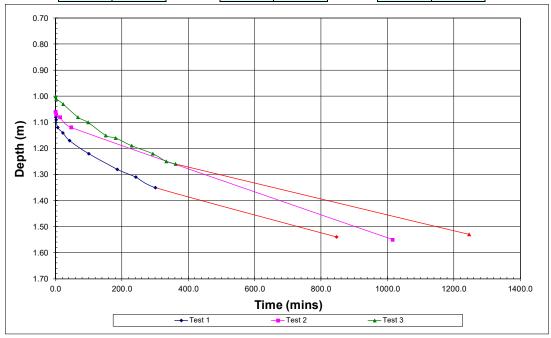
Job No. : Date : Project Name: Culham, Abingdon GE21162 Client: Statera Energy 28/09/2022 - 29/09/2022

Pit reference	TP1		
Test reference	Test1	Test2	Test3
Pit depth (m)	1.70	1.70	1.70
Pit width (m)	0.45	0.45	0.45
Pit length (m)	1.70	1.70	1.70
Depth to standing water (m)			



Test 2	
Time (min)	Depth (m)
0.0	1.06
2.0	1.07
13.0	1.08
47.0	1.12
1015.0	1.55

Test 3	
Time (min)	Depth (m)
0.0	1.00
2.0	1.01
23.0	1.03
67.0	1.08
98.0	1.10
151.0	1.15
181.0	1.16
229.0	1.19
293.0	1.22
333.0	1.25
361.0	1.26
1246.0	1.53



Max. depth (m)	1.70	1.70	1.70
Effective depth (m)	0.64	0.64	0.70
75% effective depth (m)	1.22	1.22	1.18
50% effective depth (m)	1.38	1.38	1.35
25% effective depth (m)	1.54	1.54	1.53
t75 (min)	100.00	270.00	220.00
t50 (min)	380.00	630.00	660.00
t25 (min)	847.00	1015.00	1246.00
Vp 75-25	0.24	0.24	0.27
ap 50	2.141	2.141	2.27
tp 75-25	747.00	745.00	1026.00

Soil infiltration rate (m/s) 2.6E	2.6E-06	1.9E-06
Soil infiltration rate (mm/hr) 9.18E-	9.21E+00	6.90E+00

- Blue cells require input data Infiltration calculated to method in 'BRE Digest 365 (1991) Soakaway Design' First line of table must be depth at time = 0 Extrapolated data shown in red
- 1 2 3 4



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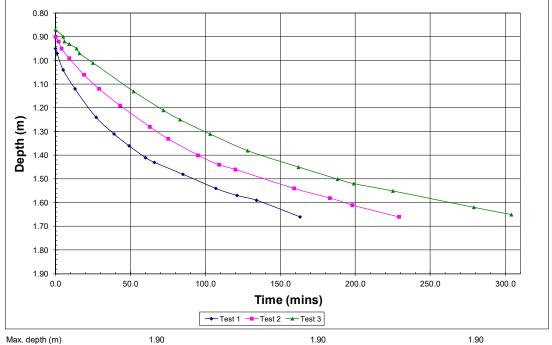
Project Name : Culham, Abingdon Client : Statera Energy Job No. : Date : GE21162 28/09/2022 - 29/09/2022

Pit reference	TP2		
Test reference	Test1	Test2	Test3
Pit depth (m)	1.90	1.90	1.90
Pit width (m)	0.45	0.45	0.45
Pit length (m)	1.30	1.30	1.30
Depth to standing water (m)			

Test 1	
Time (min)	Depth (m)
0.0	0.95
1.0	0.97
5.0	1.04
13.0	1.12
27.0	1.24
39.0	1.31
49.0	1.36
60.0	1.41
66.0	1.43
85.0	1.48
107.0	1.54
121.0	1.57
134.0	1.59
163.0	1.66

Test 2	
Time (min)	Depth (m)
0.0	0.90
2.0	0.92
4.0	0.95
9.0	0.99
19.0	1.06
29.0	1.12
43.0	1.19
63.0	1.28
75.0	1.33
95.0	1.40
109.0	1.44
120.0	1.46
159.0	1.54
183.0	1.58
198.0	1.61
229.0	1.66

Test 3	
Time (min)	Depth (m)
0.0	0.87
5.0	0.90
6.0	0.92
9.0	0.93
14.0	0.95
16.0	0.97
25.0	1.01
52.0	1.13
72.0	1.21
83.0	1.25
103.0	1.31
128.0	1.38
162.0	1.45
188.0	1.50
199.0	1.52
225.0	1.55
279.0	1.62
304.0	1.65



Max. depth (m)	1.90	1.90	1.90
Effective depth (m)	0.95	1.00	1.03
75% effective depth (m)	1.19	1.15	1.13
50% effective depth (m)	1.43	1.40	1.39
25% effective depth (m)	1.66	1.65	1.64
t75 (min)	20.00	35.00	52.00
t50 (min)	66.00	95.00	135.00
t25 (min)	163.00	220.00	295.00
Vp 75-25	0.28	0.29	0.30
ap 50	2.2475	2.335	2.3875
tp 75-25	143.00	185.00	243.00

Soil infiltration rate (m/s) 1.4E-	5 1.1E-05	8.7E-06
Soil infiltration rate (mm/hr) 5.19E+	4.06E+01	3.12E+01

- Blue cells require input data Infiltration calculated to method in 'BRE Digest 365 (1991) Soakaway Design' First line of table must be depth at time = 0
- 1 2 3



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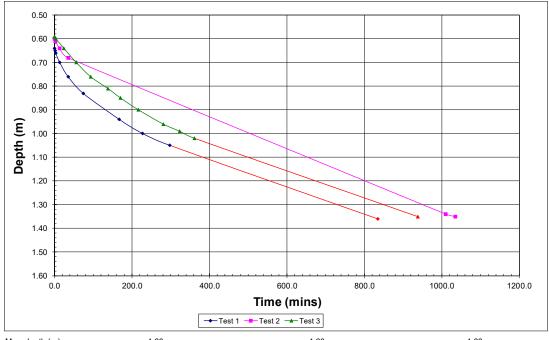
Project Name : Culham, Abingdon Client : Statera Energy Job No. : Date : GE21162 28/09/2022 - 29/09/2022

Pit reference	TP3		
Test reference	Test1	Test2	Test3
Pit depth (m)	1.60	1.60	1.60
Pit width (m)	0.45	0.45	0.45
Pit length (m)	1.60	1.60	1.60
Denth to standing water (m)			

aning water (iii)		
Test 1		
Time (min)	Depth (m)	
0.0	0.64	
1.0	0.65	
3.0	0.66	
13.0	0.70	
35.0	0.76	
74.0	0.83	
167.0	0.94	
227.0	1.00	
297.0	1.05	
835.0	1.36	

Test 2	
Time (min)	Depth (m)
0.0	0.60
2.0	0.61
13.0	0.64
35.0	0.68
1010.0	1.34
1035.0	1.35

Test 3	
Time (min)	Depth (m)
0.0	0.59
3.0	0.60
24.0	0.64
56.0	0.70
93.0	0.76
138.0	0.81
170.0	0.85
216.0	0.90
281.0	0.96
323.0	0.99
361.0	1.02
938.0	1.35



Max. depth (m)	1.60	1.60	1.60
Effective depth (m)	0.96	1.00	1.01
75% effective depth (m)	0.88	0.85	0.84
50% effective depth (m)	1.12	1.10	1.10
25% effective depth (m)	1.36	1.35	1.35
t75 (min)	190.00	290.00	160.00
t50 (min)	250.00	520.00	500.00
t25 (min)	835.00	1035.00	938.00
Vp 75-25	0.35	0.36	0.36
ap 50	2.688	2.77	2.7905
tp 75-25	645.00	745.00	778.00

Soil infiltration rate (m/s)	3.3E-06	2.9E-06	2.8E-06
Soil infiltration rate (mm/hr)	1.20E+01	1.05E+01	1.00E+01

- 1 2 3 4
- Blue cells require input data Infiltration calculated to method in 'BRE Digest 365 (1991) Soakaway Design' First line of table must be depth at time = 0 Extrapolated data shown in red



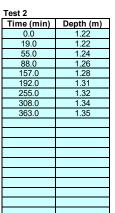
(after

Geo-EnvironmentalServices Limited Unit 7 Danworth Farm, Cuckfield Road, Hurstpierpoint, West Sussex BN6 9GL +44(0)1273 832972 www.gesl.net

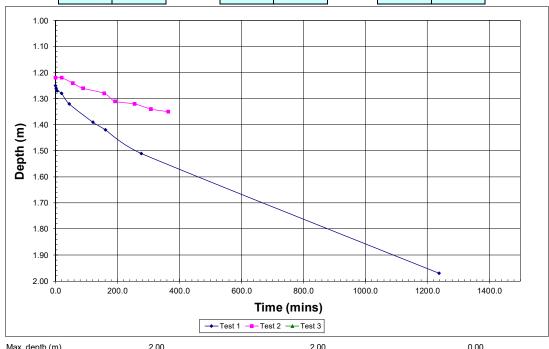
Project Name : Culham, Abingdon Client : Statera Energy Job No. : Date : GE21162 28/09/2022 - 29/09/2022

Pit reference	TP4		
Test reference	Test1	Test2	Test3
Pit depth (m)	2.00	2.00	
Pit width (m)	0.45	0.45	
Pit length (m)	1.40	1.40	
Depth to standing water (m)			

Test 1					
Depth (m)					
1.25					
1.26					
1.27					
1.28					
1.32					
1.39					
1.42					
1.51					
1.97					



Test 3 Time (min)	Depth (m)
()	



Max. depth (m)	2.00	2.00	0.00
Effective depth (m)	0.75	0.78	0.00
75% effective depth (m)	1.44	1.42	0.00
50% effective depth (m)	1.63	1.61	0.00
25% effective depth (m)	1.81	1.81	0.00
t75 (min)	180.00		
t50 (min)	530.00		
t25 (min)	900.00		
Vp 75-25	0.24	0.25	0.00
ap 50	2.0175	2.073	0
tp 75-25	720.00	0.00	0.00

Soil infiltration rate (m/s)	2.7E-06	
Soil infiltration rate (mm/hr) 9.7	76E+00	

- Blue cells require input data Infiltration calculated to method in 'BRE Digest 365 (1991) Soakaway Design' First line of table must be depth at time = 0





Discovery Strategy

Whilst an intrusive investigation has been undertaken on the site, it remains possible that unexpected ground and/or groundwater conditions may be encountered during the process of construction.

Should previously undiscovered contamination or unforeseen ground conditions be encountered during construction by the ground workers, this must be reported to the Site Manager immediately in order that the Consultant is notified.

Where deemed necessary, the Consultant shall attend the site to inspect the discovery and provide recommendations on the further actions required, if any. Where necessary the regulatory authority shall be informed. Post any additional investigation or laboratory testing the results and any proposed remedial measures shall be reported to the regulatory authority or other appropriate organisation for consent, before proceeding or implementing the remedial measures.

A copy of the discovery strategy must be lodged on site, and provisions made to ensure that all workers are made aware of their responsibility to observe, report, and act on any potentially suspicious, abnormal, unforeseen or contaminated ground and/or groundwater conditions they may encounter.

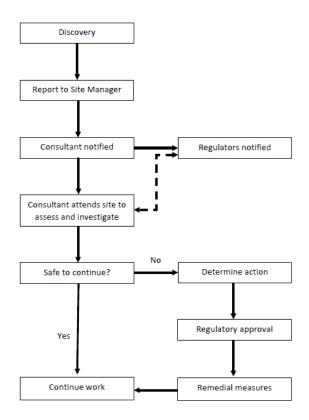
Depending on the type, nature and extent of any such 'discovery', it may be necessary to halt works in that location until such time as the assessment has been completed. This shall be reviewed on a 'discovery' specific basis and in conjunction with consultation with the client, other technical personnel and/or regulatory/approval organisations.

As a general guide, where such unexpected conditions are encountered the following approach is required as a minimum:

- All discoveries are to be reported to the Site Manager immediately and works at that location are to halt until further notice;
- The Site Manager is to report any such discoveries to the Client and the Consultant;
- Following notification from the Site Manager, the Consultant shall discuss the discovery with the Local Authority and/or other relevant parties and if considered necessary, arrange to meet on site to view the discovery;
- The Consultant shall attend the site to record the location, extent and nature of the discovery and implement an appropriate sampling and analysis regime, taking due account of the type and nature of the discovery, known and probable land uses in that area of the site;
- Where remedial action is required, regulatory consultation and approval will be sought;
- A record will be produced by the Consultant and held on site (with copies held by the Consultant, Client and Local Authority/other relevant organisation), detailing the discovery, assessment works undertaken, findings thereof, confirmation either of no action required or detailing the remedial action taken and validation thereof.

The process is summarised below:





Appendix F - Greenfield Runoff Rate

rpsgroup.com Page 31

RPS Group Plc		Page 1
Noble House, Capital Drive		
Linford Wood		
Mitlton Keynes, MK14 6QP		Micro
Date 12/03/2024 08:39	Designed by JESSICA.GRADY	Drainage
File	Checked by	Drainage
Innovyze	Source Control 2020.1	•

ICP SUDS Mean Annual Flood

Input

Return Period (years) 100 Soil 0.300
Area (ha) 4.460 Urban 0.000
SAAR (mm) 600 Region Number Region 6

Results 1/s

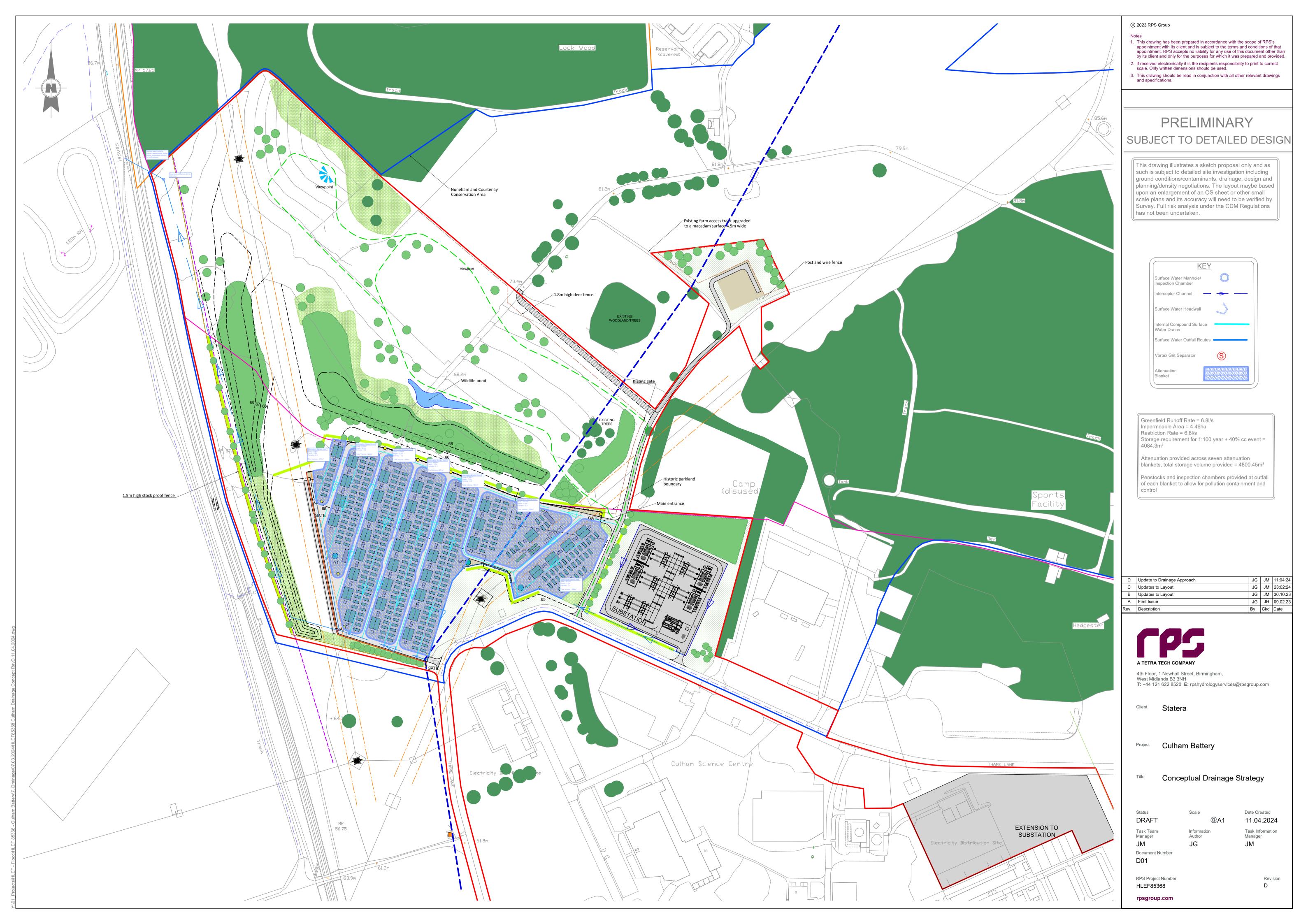
QBAR Rural 6.8 QBAR Urban 6.8

Q100 years 21.6

Q1 year 5.8 Q30 years 15.4 Q100 years 21.6

Appendix G – Conceptual Drainage Strategy

rpsgroup.com Page 32



Appendix H - MicroDrainage Calculations

rpsgroup.com Page 33

RPS Group Plc		Page 1
Noble House, Capital Drive		
Linford Wood		
Mitlton Keynes, MK14 6QP		Micro
Date 12/03/2024 13:45	Designed by JESSICA.GRADY	Drainage
File	Checked by	Dialilade
Innovyze	Source Control 2020.1	

Summary of Results for 100 year Return Period (+40%)

Half Drain Time : 5045 minutes.

	Storm	Max	Max	Max	Max	Max	Max	Status
	Event	Level	Depth	Infiltration	Control	Σ Outflow	Volume	
		(m)	(m)	(1/s)	(1/s)	(1/s)	(m³)	
15	min Sumr	ner 65.133	0.133	0.0	6.4		1281.5	O K
30	min Sumr	ner 65.174	0.174	0.0	6.8	6.8	1674.7	O K
60	min Sumr	ner 65.217	0.217	0.0	6.8	6.8	2086.1	Flood Risk
120	min Sumr	ner 65.263	0.263	0.0	6.8	6.8	2525.6	Flood Risk
180	min Summ	ner 65.291	0.291	0.0	6.8	6.8	2789.4	Flood Risk
240	min Sumr	ner 65.309	0.309	0.0	6.8	6.8	2969.3	Flood Risk
360	min Sumr	ner 65.333	0.333	0.0	6.8	6.8	3195.5	Flood Risk
480	min Sumr	ner 65.347	0.347	0.0	6.8	6.8	3333.1	Flood Risk
600	min Sumr	ner 65.357	0.357	0.0	6.8	6.8	3424.0	Flood Risk
720	min Summ	mer 65.363	0.363	0.0	6.8	6.8	3486.7	Flood Risk
960	min Summ	ner 65.371	0.371	0.0	6.8	6.8	3560.7	Flood Risk
1440	min Summ	ner 65.376	0.376	0.0	6.8	6.8	3613.9	Flood Risk
2160	min Summ	ner 65.375	0.375	0.0	6.8	6.8	3604.7	Flood Risk
2880	min Summ	ner 65.371	0.371	0.0	6.8	6.8	3557.7	Flood Risk
4320	min Sumr	ner 65.357	0.357	0.0	6.8	6.8	3428.5	Flood Risk
5760	min Sumn	ner 65.348	0.348	0.0	6.8	6.8	3339.4	Flood Risk
7200	min Summ	ner 65.343	0.343	0.0	6.8	6.8	3289.7	Flood Risk
		ner 65.339		0.0	6.8			Flood Risk
		ner 65.337		0.0	6.8			Flood Risk
		ter 65.150		0.0	6.7		1435.4	0 K
10	***		3.130	0.0	J . /	0.7	_ 100.1	0 10

	Stor Even		Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
15	min	Summer	153.813	0.0	439.0	27
30	min	Summer	100.662	0.0	554.6	42
60	min	Summer	62.851	0.0	1100.0	72
120	min	Summer	38.216	0.0	1145.1	132
180	min	Summer	28.256	0.0	1132.6	192
240	min	Summer	22.648	0.0	1117.6	250
360	min	Summer	16.376	0.0	1088.8	370
480	min	Summer	12.909	0.0	1062.7	490
600	min	Summer	10.688	0.0	1038.7	610
720	min	Summer	9.138	0.0	1016.5	730
960	min	Summer	7.104	0.0	975.8	968
1440	min	Summer	4.953	0.0	904.1	1448
2160	min	Summer	3.446	0.0	1950.0	2164
2880	min	Summer	2.670	0.0	1848.5	2884
4320	min	Summer	1.879	0.0	1658.4	4020
5760	min	Summer	1.476	0.0	3739.2	4568
7200	min	Summer	1.235	0.0	3604.0	5328
8640	min	Summer	1.075	0.0	3415.8	6128
10080	min	Summer	0.960	0.0	3225.7	6952
15	min	Winter	153.813	0.0	496.8	27

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RPS Group Plc		Page 2
Noble House, Capital Drive		
Linford Wood		
Mitlton Keynes, MK14 6QP		Micro
Date 12/03/2024 13:45	Designed by JESSICA.GRADY	Drainage
File	Checked by	Dialilade
Innovyze	Source Control 2020.1	

Summary of Results for 100 year Return Period (+40%)

	Storm Event		Max Level (m)	Max Depth (m)	Max Infiltration (1/s)	Max Control (1/s)	Max Σ Outflow (1/s)	Max Volume (m³)	Status
30	min Wir	nter	65.195	0.195	0.0	6.8	6.8	1876.4	O K
60	min Wir	nter	65.244	0.244	0.0	6.8	6.8	2337.8	Flood Risk
120	min Wir	nter	65.295	0.295	0.0	6.8	6.8	2831.5	Flood Risk
180	min Wir	nter	65.326	0.326	0.0	6.8	6.8	3128.4	Flood Risk
240	min Wir	nter	65.347	0.347	0.0	6.8	6.8	3331.3	Flood Risk
360	min Wir	nter	65.374	0.374	0.0	6.8	6.8	3587.8	Flood Risk
480	min Wir	nter	65.390	0.390	0.0	6.8	6.8	3745.2	Flood Risk
600	min Wir	nter	65.401	0.401	0.0	6.8	6.8	3849.7	Flood Risk
720	min Wir	nter	65.409	0.409	0.0	6.8	6.8	3922.4	Flood Risk
960	min Wir	nter	65.418	0.418	0.0	6.8	6.8	4009.8	Flood Risk
1440	min Wir	nter	65.425	0.425	0.0	6.8	6.8	4078.4	Flood Risk
2160	min Wir	nter	65.425	0.425	0.0	6.8	6.8	4084.3	Flood Risk
2880	min Wir	nter	65.422	0.422	0.0	6.8	6.8	4051.4	Flood Risk
4320	min Wir	nter	65.411	0.411	0.0	6.8	6.8	3950.2	Flood Risk
5760	min Wir	nter	65.399	0.399	0.0	6.8	6.8	3832.8	Flood Risk
7200	min Wir	nter	65.390	0.390	0.0	6.8	6.8	3743.7	Flood Risk
8640	min Wir	nter	65.384	0.384	0.0	6.8	6.8	3687.9	Flood Risk
10080	min Wir	nter	65.379	0.379	0.0	6.8	6.8	3637.9	Flood Risk

Storm			Rain	Flooded	Discharge	Time-Peak
Event		(mm/hr)	Volume	Volume	(mins)	
				(m³)	(m³)	
20		T-7	100 660	0 0	E74 4	41
			100.662	0.0	574.4	
		Winter	62.851	0.0	1148.0	72
120	min	Winter	38.216	0.0	1142.3	130
180	min	Winter	28.256	0.0	1122.1	188
240	min	Winter	22.648	0.0	1102.0	248
360	min	Winter	16.376	0.0	1064.8	366
480	min	Winter	12.909	0.0	1032.6	484
600	min	Winter	10.688	0.0	1008.8	602
720	min	Winter	9.138	0.0	989.7	720
960	min	Winter	7.104	0.0	958.4	956
1440	min	Winter	4.953	0.0	906.6	1426
2160	min	Winter	3.446	0.0	1934.2	2120
2880	min	Winter	2.670	0.0	1844.1	2804
4320	min	Winter	1.879	0.0	1677.2	4148
5760	min	Winter	1.476	0.0	3797.0	5368
7200	min	Winter	1.235	0.0	3612.3	5832
8640	min	Winter	1.075	0.0	3435.2	6672
10080	min	Winter	0.960	0.0	3278.3	7656

RPS Group Plc	Page 3	
Noble House, Capital Drive		
Linford Wood		
Mitlton Keynes, MK14 6QP		Micro
Date 12/03/2024 13:45	Designed by JESSICA.GRADY	Drainage
File	Checked by	nialilade
Innovyze	Source Control 2020.1	'

Rainfall Details

Rainfall Model						FEH
Return Period (years)						100
FEH Rainfall Version						2013
Site Location	GB	452975	196383	SU	52975	96383
Data Type						Point
Summer Storms						Yes
Winter Storms						Yes
Cv (Summer)						0.750
Cv (Winter)						0.840
Shortest Storm (mins)						15
Longest Storm (mins)						10080
Climate Change %						+40

Time Area Diagram

Total Area (ha) 4.460

							(mins)	
From:	To:	(ha)	From:	To:	(ha)	From:	To:	(ha)
0	4	1.487	4	8	1.487	8	12	1.487

RPS Group Plc		Page 4
Noble House, Capital Drive		
Linford Wood		
Mitlton Keynes, MK14 6QP		Micro
Date 12/03/2024 13:45	Designed by JESSICA.GRADY	Drainage
File	Checked by	Dialilade
Innovyze	Source Control 2020.1	

Model Details

Storage is Online Cover Level (m) 65.500

Infiltration Blanket Structure

Infiltration Coefficient Base (m/hr) 0.00000 Diameter/Width (m) 100.0 Safety Factor 2.0 Length (m) 320.0 Porosity 0.30 Cap Volume Depth (m) 0.000 Invert Level (m) 65.000

Hydro-Brake® Optimum Outflow Control

Unit Reference MD-SHE-0129-6800-0500-6800 0.500 Design Head (m) Design Flow (1/s) 6.8 Flush-Flo™ Calculated Objective Minimise upstream storage Application Surface Sump Available Yes Diameter (mm) 129 Invert Level (m) 65.000 Minimum Outlet Pipe Diameter (mm) 150 Suggested Manhole Diameter (mm) 1200

Control Points Head (m) Flow (1/s) Design Point (Calculated) 0.500 6.8 Flush-Flo™ 0.198 6.8 Kick-Flo® 0.382 6.0 Mean Flow over Head Range 5.5

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m) Flo	ow (1/s)	Depth (m) Flow	(1/s)	Depth (m) Flow	(1/s)	Depth (m)	Flow (1/s)
0.100	4.6	1.200	10.3	3.000	15.8	7.000	23.8
0.200	6.8	1.400	11.0	3.500	17.1	7.500	24.7
0.300	6.6	1.600	11.7	4.000	18.2	8.000	25.5
0.400	6.1	1.800	12.4	4.500	19.2	8.500	26.3
0.500	6.8	2.000	13.1	5.000	20.2	9.000	27.1
0.600	7.4	2.200	13.7	5.500	21.1	9.500	27.8
0.800	8.5	2.400	14.2	6.000	22.1		
1.000	9.4	2.600	14.8	6.500	23.0		

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